CONCEPT CIVIL ENGINEERING PLANS

WOOLWORTHS, KURRI KURRI LOT 136 DP 869710 174-178 LANG STREET, KURRI KURRI

NOTES

1. THESE DRAWINGS ARE TO BE READ IN CONJUNCTION WITH THE CONDITIONS STATED IN COUNCIL'S DEVELOPMENT APPROVAL AND THE APPROVED

SHEET LIST

SHEET NO.	SHEET TITLE
01	COVER SHEET
02	DETAIL PLAN
03	EROSION & SEDIMENT CONTROL PLAN
04	EROSION & SEDIMENT CONTROL DETAILS
05	TYPICAL SECTIONS & DETAILS
06	ROAD PROFILES
07	ONSITE DETENTION CALCULATIONS
08	STORMWATER QUALITY TREATMENT CALCULATIONS
09	VEHICLE MOVEMENTS PLAN

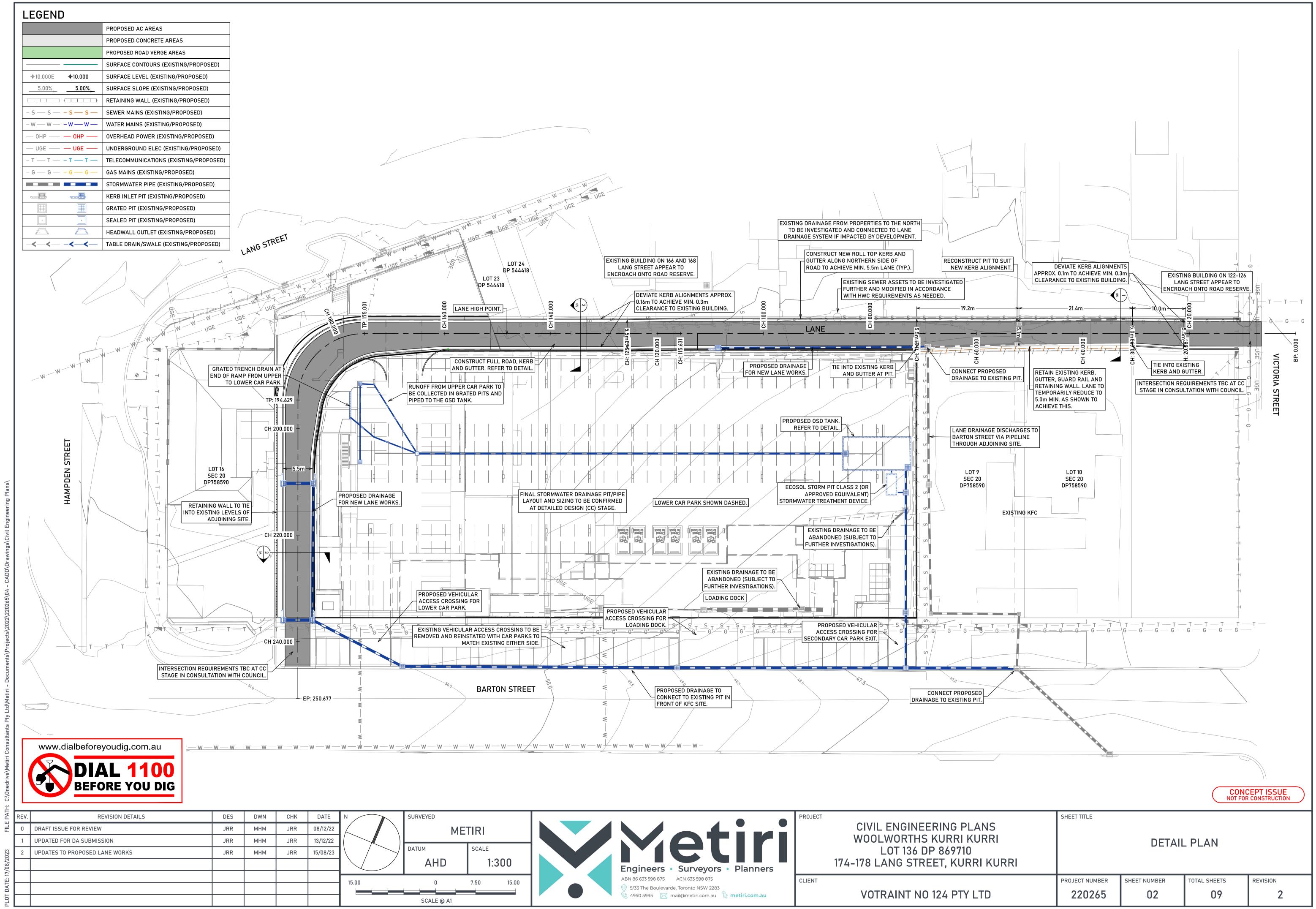


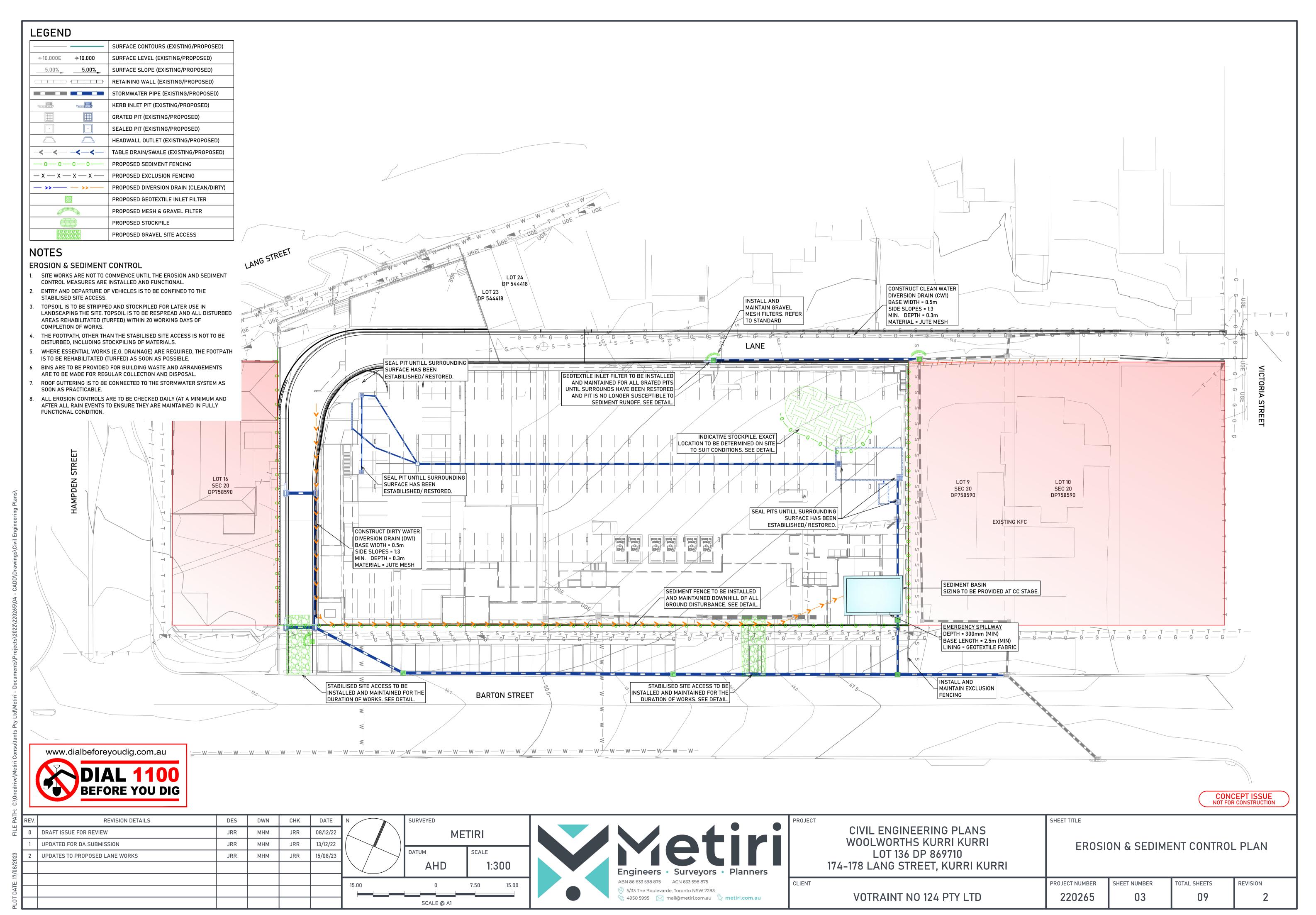
LOCALITY PLAN

REV.	REVISION DETAILS	DES	DWN	СНК	DATE	N	SURVEYED			
0	DRAFT ISSUE FOR REVIEW	JRR	МНМ	JRR	08/12/22		METIRI			
1	UPDATED FOR DA SUBMISSION	JRR	МНМ	JRR	13/12/22					
2	UPDATES TO PROPOSED LANE WORKS	JRR	МНМ	JRR	15/08/23		DATUM	SCALE		
							AHD			



PROJECT	CIVIL ENGINEERING PLANS WOOLWORTHS KURRI KURRI LOT 136 DP 869710 174-178 LANG STREET, KURRI KURRI	SHEET TITLE	COVER	SHEET	
LIENT		PROJECT NUMBER	SHEET NUMBER	TOTAL SHEETS	REVISION
	VOTRAINT NO 124 PTY LTD	220265	01	09	2





SEDIMENT FENCE (SD 6-8)

- CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER SECOND IN THE DESIGN STORM EVENT, USUALLY THE 10-YEAR EVENT.
- 2. CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED.
- DRIVE 1.5m LONG STAR PICKETS INTO GROUND AT 2.5m INTERVALS (MAX) AT THE DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
- FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- 5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP. 6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

GEOTEXTILE INLET FILTER (SD 6-12)

- FABRICATE A SEDIMENT BARRIER MADE FROM GEOTEXTILE OR STRAW
- 2. FOLLOW STANDARD DRAWING 6-7 AND STANDARD DRAWING 6-8 FOR INSTALLATION PROCEDURES FOR THE STRAW BALES OR GEOFABRIC. REDUCE THE PICKET SPACING TO 1m CENTRES.
- 3. IN WATERWAYS, ARTIFICIAL SAG POINTS CAN BE CREATED WITH SANDBAGS OR EARTH BANKS AS SHOWN IN THE DRAWING.
- 4. DO NOT COVER THE INLET WITH GEOTEXTILE UNLESS THE DESIGN IS ADEQUATE TO ALLOW FOR ALL WATERS TO BYPASS IT.

STOCKPILES (SD4-1)

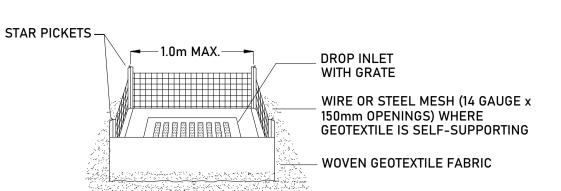
- PLACE STOCKPILES MORE THAN 2 (PREFERABLY 5) METRES FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW, ROADS AND HAZARD AREAS.
- 2. CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.
- 3. WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2m IN HEIGHT.
- WHERE THEY ARE TO BE IN PLACE FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE APPROVED ESCP OR SWMP TO REDUCE THE C-FACTOR TO LESS THAN 0.10.
- CONSTRUCT EARTH BANKS (STANDARD DRAWING 5-5) ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDIMENT FENCES (STANDARD DRAWING 6-8) 1m TO 2m DOWNSLOPE.

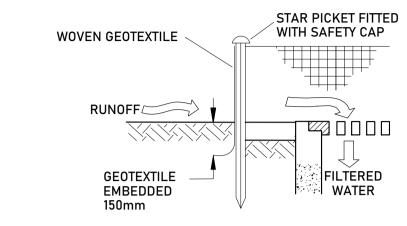
STABILISED SITE ACCESS (SD6-14)

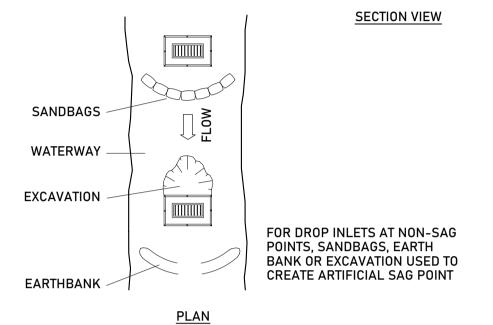
- 1. STRIP THE TOPSOIL, LEVEL THE SITE AND COMPACT THE SUBGRADE.
- 2. COVER THE AREA WITH NEEDLE-PUNCHED GEOTEXTILE.
- CONSTRUCT A 200mm THICK PAD OVER THE GEOTEXTILE USING ROAD BASE OR 30 MM AGGREGATE.
- 4. ENSURE THE STRUCTURE IS AT LEAST 15 METRES LONG OR TO BUILDING ALIGNMENT AND AT LEAST 3 METRES WIDE.
- WHERE A SEDIMENT FENCE JOINS ONTO THE STABILISED ACCESS, CONSTRUCT A HUMP IN THE STABILISED ACCESS TO DIVERT WATER TO THE SEDIMENT FENCE.

MESH AND GRAVEL INLET FILTER (SD6-11)

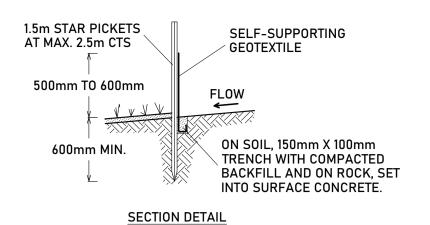
- 1. INSTALL FILTERS TO KERB INLETS ONLY AT SAG POINTS.
- 2. FABRICATE A SLEEVE MADE FROM GEOTEXTILE OR WIRE MESH LONGER THAN THE LENGTH OF THE INLET PIT AND FILL IT WITH 25mm TO 50mm
- 3. FORM AN ELLIPTICAL CROSS-SECTION ABOUT 150mm HIGH X 400mm WIDE PLACE THE FILTER AT THE OPENING LEAVING AT LEAST A 100-MM SPACE BETWEEN IT AND THE KERB INLET. MAINTAIN THE OPENING WITH SPACER
- FORM A SEAL WITH THE KERB TO PREVENT SEDIMENT BYPASSING THE
- SANDBAGS FILLED WITH GRAVEL CAN SUBSTITUTE FOR THE MESH OR GEOTEXTILE PROVIDING THEY ARE PLACED SO THAT THEY FIRMLY ABUT EACH OTHER AND SEDIMENT-LADEN WATERS CANNOT PASS BETWEEN

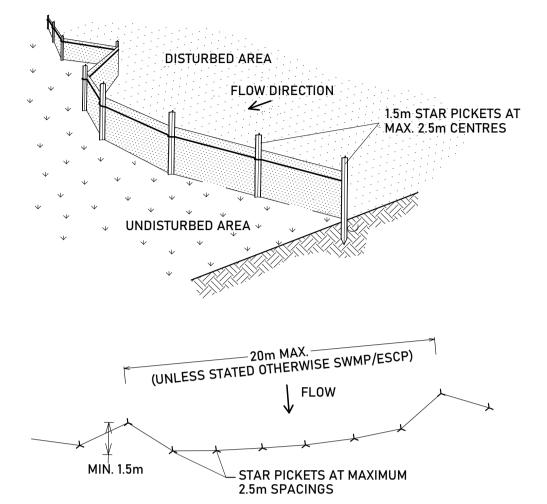




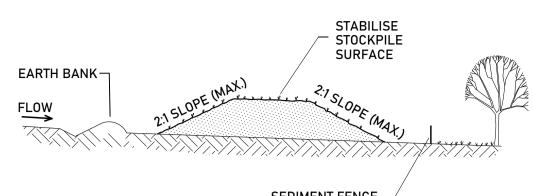


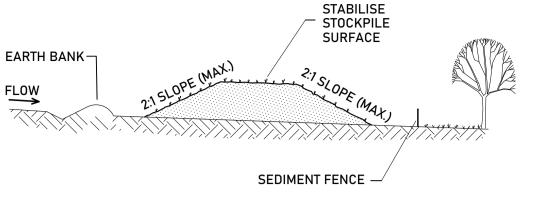
GEOTEXTILE INLET FILTER (SD 6-12) DETAIL

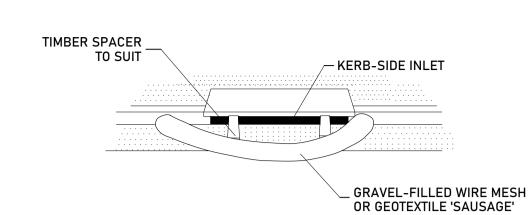




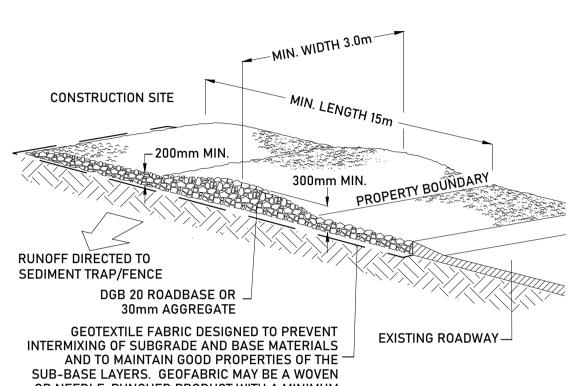
SEDIMENT FENCE (SD 6-8) DETAIL



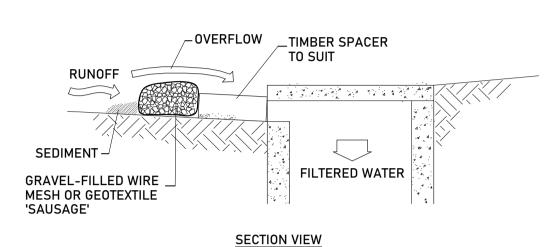








OR NEEDLE-PUNCHED PRODUCT WITH A MINIMUM CBR BURST STRENGTH (AS3706.4-90) OF 2500N STABILISED SITE ACCESS (SD 6-14) DETAIL



NOTE: THIS PRACTICE IS ONLY TO BE USED WHERE SPECIFIED IN AN APPROVED SWMP/ESCP.

MESH AND GRAVEL INLET FILTER (SD 6-11) DETAIL

CONCEPT ISSUE NOT FOR CONSTRUCTION

REVISION DETAILS DES DWN DATE CHK 0 DRAFT ISSUE FOR REVIEW **JRR** UPDATED FOR DA SUBMISSION **JRR** 13/12/22 MHM UPDATES TO PROPOSED LANE WORKS JRR MHM JRR 15/08/23

METIRI SCALE

SURVEYED

DATUM

Engineers • Surveyors • Planners ABN 86 633 598 875 ACN 633 598 875 (9) 5/33 The Boulevarde, Toronto NSW 2283 4950 5995 mail@metiri.com.au netiri.com.au

CIVIL ENGINEERING PLANS WOOLWORTHS KURRI KURRI LOT 136 DP 869710 174-178 LANG STREET, KURRI KURRI

EROSION & SEDIMENT CONTROL DETAILS

SHEET NUMBER PROJECT NUMBER TOTAL SHEETS REVISION **VOTRAINT NO 124 PTY LTD** 220265 09 04

SHEET TITLE

CLIENT

ROAD PAVEMENT

- 1. ROAD PAVEMENT CONSTRUCTION IS TO BE CARRIED OUT IN ACCORDANCE WITH THE APPROVED GEOTECHNICAL REPORT. IF THE STANDARD OR REQUIREMENTS FOR WORK SHOWN ON THE DRAWINGS DIFFER FROM THAT REQUIRED BY THE APPROVED GEOTECHNICAL REPORT, THE REQUIREMENTS OF THE GEOTECHNICAL REPORT WILL GENERALLY PREVAIL. CLARIFICATION SHALL BE SOUGHT FROM METIRI SHOULD DISCREPANCIES ARISE.
- INSPECTION SHOULD BE CARRIED OUT BY A GEOTECHNICAL AUTHORITY DURING CONSTRUCTION TO VERIFY THE CONDITIONS ASSUMED IN THE GEOTECHNICAL REPORT AND IN THE DESIGN.

KERB & GUTTER

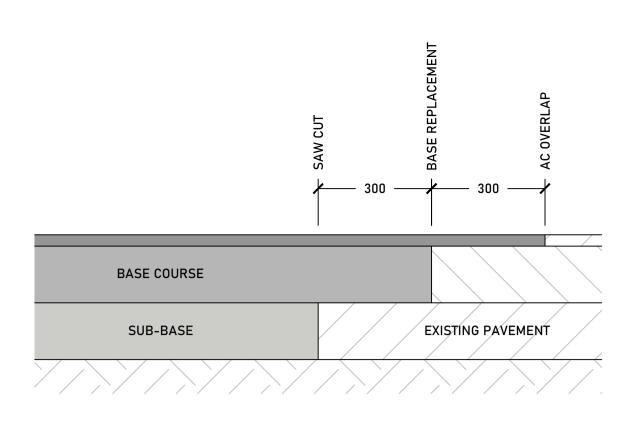
- 1. ALL EXPOSED KERB EDGES ARE TO BE ROUNDED TO 20mm RADIUS.
- 2. ALL KERB IS TO BE CONSTRUCTED WITH 25MPa CONCRETE.
- 3. CONTROL JOINTS ARE TO BE PROVIDED EVERY 2.5-3.0m AND EXPANSION JOINTS TO BE PROVIDED EVERY 15m AND AT EVERY LAYBACK, LINTEL AND KERB RAMP.

SUBSOIL DRAINAGE

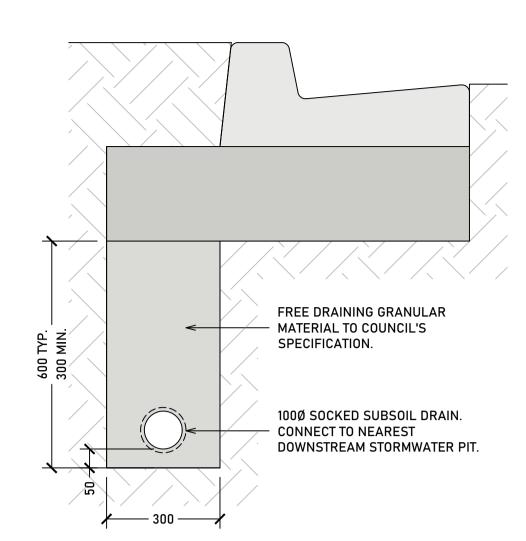
- MINIMUM SUBSOIL TRENCH DEPTH IS TO BE 600MM IN EARTH AND 450MM IN ROCK.
- 2. TRENCHES ARE TO BE LOCATED BELOW INVERT LEVEL OF ANY SERVICE CROSSINGS.
- 3. PAVEMENT LAYERS ARE TO EXTEND TO AT LEAST THE LINE OF THE REAR OF

MINIMUM LONGITUDINAL GRADE IS 1.0%. FOR NON-CORRUGATED PIPES. AN

ABSOLUTE MINIMUM GRADE OF 0.5% IS ACCEPTABLE. 5. CLEANOUT POINTS ARE TO BE PROVIDED AT THE COMMENCEMENT OF EACH RUN OF SUBSOIL DRAINAGE LINE AND AT INTERVALS NOT EXCEEDING 80M.



DETAIL - TYPICAL PAVEMENT CONNECTION



DETAIL - SUBSOIL DRAINAGE

30 // 150 // 30

PROPOSED LANE - SECTION 1

- 6.235m ROAD RESERVE —

- 5.0m CARRIAGEWAY -

PROPOSED ROLL TOP KERB

AND GUTTER. SEE DETAIL.

SUBSOIL DRAIN. SEE DETAIL.

0.45m 😽

 \longrightarrow

← 0.85m →

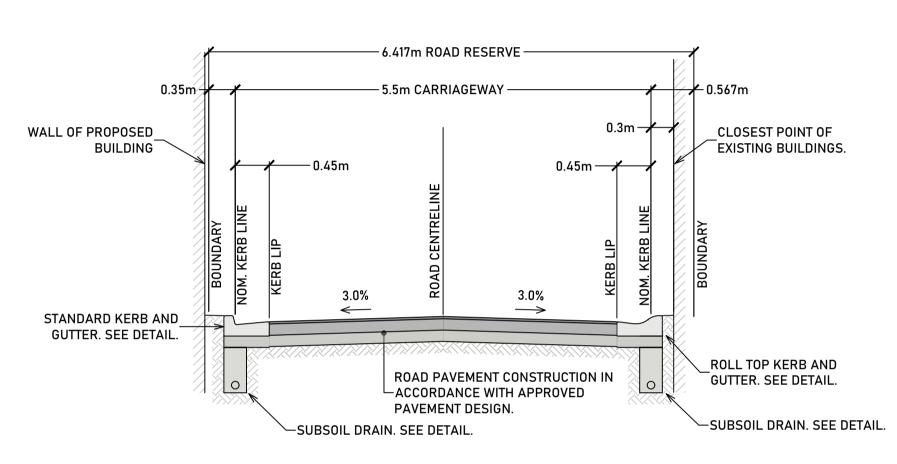
ROAD PAVEMENT RECONSTRUCTION

IN ACCORDANCE WITH APPROVED-

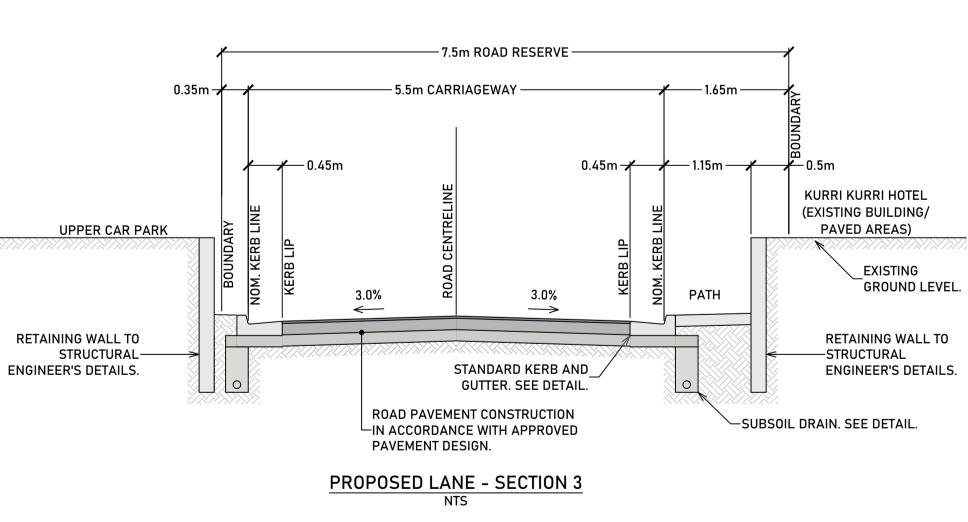
PAVEMENT DESIGN.

EXISTING KERB, GUTTER

AND GUARD RAIL.



PROPOSED LANE - SECTION 2



DETAIL - STANDARD KERB & GUTTER

CONCRETE STRENGTH SL82 MESH GRADE TO BE N32 ON -60 BOTTOM COVER COMPACTED SUBBASE

DETAIL - DISHED CROSSING (MODIFIED SB)

PROJECT

CLIENT

CONCEPT ISSUE NOT FOR CONSTRUCTION



	PROVIDE RISERS FOR ACCESS REQUIRED TO SUIT FINISHED S		BOLT DOWN INSPECTION AND MAINTENANCE GRATE. RL=50.4
		V	LIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\			
	BOLT DOWN INSPECTION	↑	RL=49.45
	AND MAINTENANCE GRATE.		TRASH SCREEN. — Ø225 OVERFLOW PIPE TO
		_ E	BYPASS TREATMENT UNIT.
8		- 2.0	Ø180 ORIFICE PLATE. Ø300 PIPE TO NOMINATED
	INCOMING PIPE.		TREATMENT UNIT.
	REFER TO PLAN.	WEEP HOLES IN BA	ASE OF TANK.
/\/,			200mm OF CLEAN, FREE DRAINING AGGREGATE (10-20mm) WRAPPED IN GEOFABRIC.
		75m ²	

DETAIL - CONCRETE DETENTION TANK

REV.	REVISION DETAILS	DES	DWN	СНК	DATE	N
0	DRAFT ISSUE FOR REVIEW	JRR	МНМ	JRR	08/12/22	
1	UPDATED FOR DA SUBMISSION	JRR	МНМ	JRR	13/12/22	
2	UPDATES TO PROPOSED LANE WORKS	JRR	МНМ	JRR	15/08/23	

SURVEYED	TIDI
ME	TIRI
DATUM	SCALE
AHD	

	Matiri
	Engineers • Surveyors • Planners
٦	ABN 86 633 598 875 ACN 633 598 875
	5/33 The Boulevarde, Toronto NSW 22834950 5995 mail@metiri.com.au metiri.com.au

CIVIL ENGINEERING PLANS WOOLWORTHS KURRI KURRI LOT 136 DP 869710 I74-178 LANG STREET, KURRI KURRI	SHEET TITLE	PICAL SECTION	ONS & DETAI	LS
	PROJECT NUMBER	SHEET NUMBER	TOTAL SHEETS	REVISION
VOTRAINT NO 124 PTY LTD	220265	05	09	2

REVISION DETAILS DES SURVEYED DWN CHK DATE **METIRI** 0 DRAFT ISSUE FOR REVIEW 1 UPDATED FOR DA SUBMISSION MHM JRR 13/12/22 DATUM JRR 15/08/23 UPDATES TO PROPOSED LANE WORKS 1:500 25.00 12.50 25.00 SCALE @ A1



CIVIL ENGINEERING PLANS WOOLWORTHS KURRI KURRI LOT 136 DP 869710 174-178 LANG STREET, KURRI KURRI

ROAD PROFILES CLIENT SHEET NUMBER TOTAL SHEETS PROJECT NUMBER REVISION 220265 09 06

SHEET TITLE

CONCEPT ISSUE NOT FOR CONSTRUCTION

								1.0%		K=0	66.2 25.0		5.0	1%		NC 30. S = 10 CH 138.126 O = 20.034	CREST CH 148.155 CREST RL 53.910	->	-1	0%			-1.0%		N S S S S S S S S S S S S S S S S S S S	I.7 15.0	-10.0%	K=	2.0 15.0	-2.5%	2.8%	
DATUM 45.000																																
PROPOSED LEVELS							51.653	51.769	51.882	52.019	52.177	52.519 52.628	52.910	53.128	53.403	53.844	53.906	53.885	53.817	53.718	53.668	53.618	53.515	53.417	53.412	52.986	52.049	51.308	51.238	50.979 50.888 50.888	50.802 50.762 50.802	50.959
EXISTING LEVELS	52.411	52.057	51.801	51.657	51.555	51.517	51.629	51.643	51.877	51.989	52.136	52.552 52.681	52.942	53.143	53.363	53.720	54.005													50.983	50.839 50.816 50.788	50.947
LEVEL DIFFERENCE							0.012	0.098	0.005	0.030	0.041	-0.033	-0.032	-0.015	0.041	0.089	-0.099													-0.095	-0.037 -0.054 0.014	0.012
CHAINAGE	10.000	20.000	30.000	40.000	50.000	90.000	70.000	80.000	00.00	95.327	100.000	107.819	115.631	_		138.126	150.000	153.135	160.000	170.000	175.001	180.000	190.000	199.581	200.000	210.000	220.000	221.337	230.000	236.337 240.000	243.891 244.341	250.000

LANE-CL PROFILE (5x VERTICAL EXAGGERATION)

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CATCHMENTS

THE PRE-DEVELOPED SITE CONSISTS OF A SINGLE CATCHMENT CONTRIBUTING STORMWATER RUNOFF IN A SOUTH-EASTERLY DIRECTION TOWARDS BARTON STREET. PRE-DEVELOPED CATCHMENT PROPERTIES ARE PROVIDED IN THE TABLE BELOW.

PRE-DEVELOPED CATCHMENT PROPERTIES

CATCHMENT	SITE				
AREA - TOTAL (ha)	0.570				
AREA - IMPERVIOUS (ha)	0.000				
AREA - PERVIOUS (ha)	0.570				
% IMPERVIOUS	0.0				
AVERAGE SLOPE (%)	5.0				

ONCE DEVELOPED AS PROPOSED, THE SITE WILL GENERALLY RETAIN THE PRE-DEVELOPED CATCHMENTS ALTHOUGH WITH ALTERED PROPERTIES.

POST-DEVELOPED CATCHMENT PROPERTIES ARE PROVIDED IN THE TABLE BELOW.

POST-DEVELOPED CATCHMENT PROPERTIES

CATCHMENT	TO OSD
AREA - TOTAL (ha)	0.570
AREA - IMPERVIOUS (ha)	0.557
AREA - PERVIOUS (ha)	0.013
% IMPERVIOUS	97.7
AVERAGE SLOPE (%)	1.0

HYDROLOGICAL ANALYSIS

A PRELIMINARY HYDROLOGICAL MODEL WAS DEVELOPED USING INFOWORKS ICM SOFTWARE. STORM ENSEMBLES, TYPICALLY CONSISTING OF 10 STORMS FOR EACH AEP FOR EACH NOMINATED DURATION, WERE PRODUCED USING THE ARR STORM GENERATOR. FROM HERE, THE ENSEMBLE STATISTICS TOOL WAS USED TO DETERMINE THE STATISTICAL MEAN AND MEDIAN STORM EVENTS, ALONG WITH ANY OUTLIERS. THE LARGEST MEDIAN STORM EVENT FOR EACH AEP IS THEN USED AS THE DESIGN STORM FOR THAT AEP.

AEP'S SELECTED FOR THE ANALYSIS WERE THE 20%, 10%, 5% AND 1%. THESE ARE CONSIDERED A SUITABLE RANGE OF EVENTS FOR SIZING OF MINOR AND MAJOR HYDRAULIC STRUCTURES INCLUDING STORMWATER DETENTION FACILITIES.

THE BOX AND WHISKER PLOTS PRODUCED USING THE ENSEMBLE STATISTICS TOOL ARE PROVIDED TO THE RIGHT. THE VALUES IN GREEN ARE THE PEAK DISCHARGE VALUES ASSOCIATED WITH THE LARGEST MEDIAN STORM EVENT FOR EACH AEP.

ONSITE DETENTION (OSD)

THE PROPOSED DEVELOPMENT IS REQUIRED TO ATTENUATE PEAK POST-DEVELOPED FLOW RATES TO LESS THAN OR EQUAL TO PRE-DEVELOPED RATES. IT IS PROPOSED TO ACHIEVE THIS BY ROUTING STORMWATER THROUGH AN UNDERGROUND DETENTION TANK LOCATED NEAR THE LOW POINT ON THE SITE, WITHIN THE LOWER CAR PARK.

A SIMPLIFIED HYDRAULIC MODEL WAS CREATED IN ICM TO DETERMINE THE REQUIRED STORAGE VOLUME AND OUTLET CONFIGURATION. A SCREENSHOT OF THE MODEL IS SHOWN TO THE RIGHT. THE MODEL ON THE LEFT REPRESENTS THE PRE-DEVELOPED SCENARIO WITH THE MODEL ON THE RIGHT REPRESENTING THE POST-DEVELOPED SCENARIO.

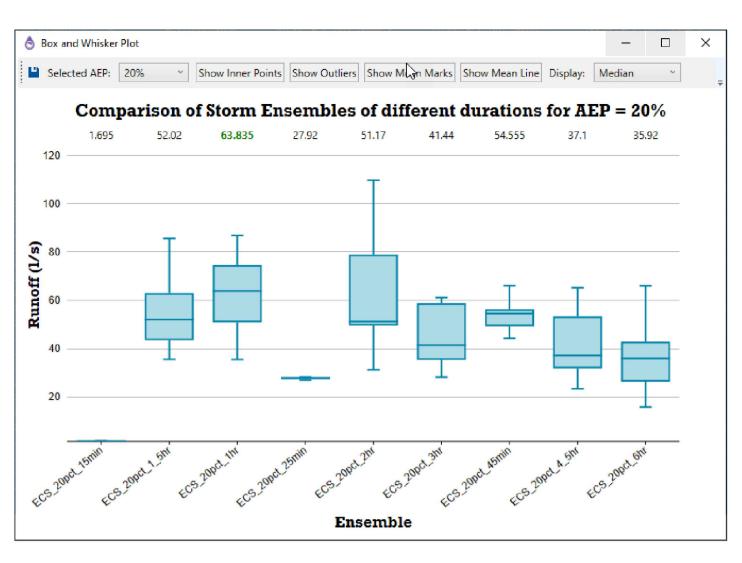
SEVERAL CONFIGURATIONS OF THE OSD SYSTEM WERE TESTED, WITH THE OPTIMAL CONFIGURATION TO MEET THE REQUIRED FLOW ATTENUATION DETAILED ON THE PREVIOUS SHEET(S).

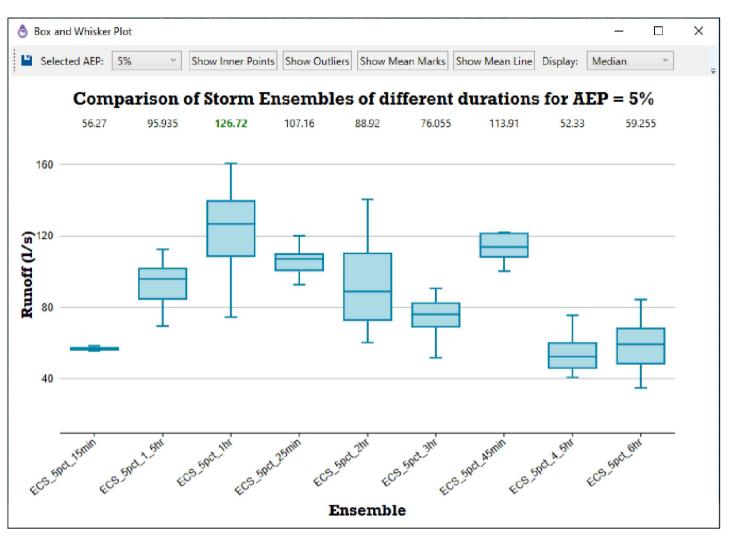
RESULTS FROM THE OSD MODEL ARE SUMMARISED IN THE TABLE BELOW.
OSD MODEL RESULTS.

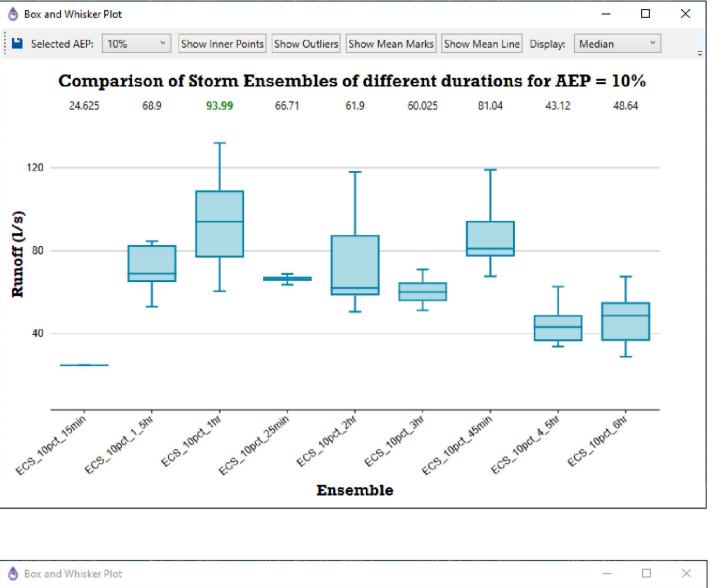
AEP	PEAK FL	0W (L/s)				
AEP	PRE-DEVELOPED	POST-DEVELOPED				
20%	68.5	60.6				
10%	96.4	95.1				
5%	130.3	102.2				
1%	206.8	181.8				

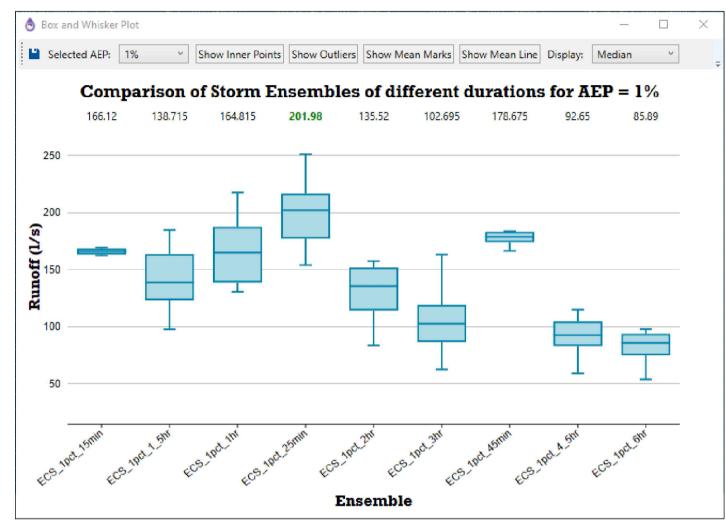
RESULTS FROM THE MODEL SHOW THAT BY ROUTING STORMWATER RUNOFF THROUGH THE PROPOSED OSD SYSTEM, PEAK FLOWS ASSOCIATED WITH THE POST-DEVELOPED SITE ARE LESS THAN OR EQUAL TO THE PEAK FLOWS ASSOCIATED WITH THE PRE-DEVELOPED SITE.

THE PRE-DEVELOPED VS. POST-DEVELOPED SITE OUTFLOW HYDROGRAPHS FOR THE CRITICAL 20%, 10%, 5% AND 1% AEP STORM EVENTS ARE PROVIDED TO THE RIGHT. THE GREEN CURVE INDICATES THE PRE-DEVELOPED SCENARIO WITH THE RED CURVE INDICATING THE DEVELOPED SCENARIO.







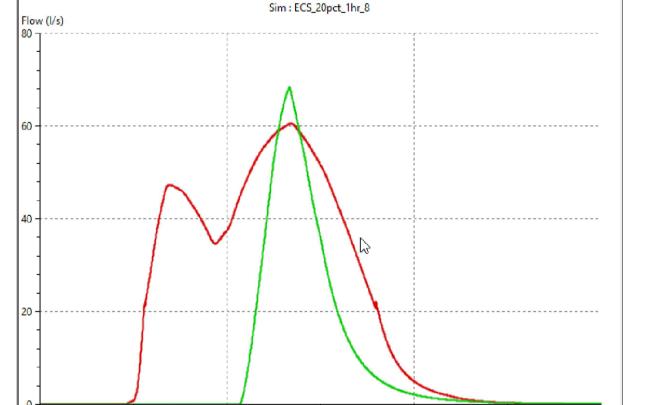


Post-Developed Lot Predevelopment ID Post-Developed Lot PERV 0.013/IMP 0.557 Total area (ha) 0.570 PERV 0.570/IMP 0.000 Total area (ha) 0.570 Slope (%) Slope (%) max runoff (I/s) 206.80 max runoff (l/s) 269.90 max level (m AD) 50.263 max inflow (I/s) 269.90 max volume (m3) 152.7 Asset ID Low Orifice Invert level (m AD) 48.200 Asset ID Overflow Pipe Diameter (m) Invert level (m AD) 49.450 max us_depth (m) 2.063 Diameter (m) 0.225 Predev.1 max us_flow (I/s) 92.10 max us_depth (m) 0.813 max us_flow (I/s) 89.61 max us_flow (I/s) 206.80 max us_vel (m/s) 3.828 Volume (m3) Asset ID Control Pit Surcharge Crest (m AD) max us_flow (I/s) 0.00 width (mm) max us_flow (I/s) 181.71 max us_vel (m/s) 3.326 Predev-outfall Volume (m3) Post Outfall ICM MODEL (20% RESULTS DISPLAYED)

SHEET TITLE

PROJECT NUMBER

220265



02:00

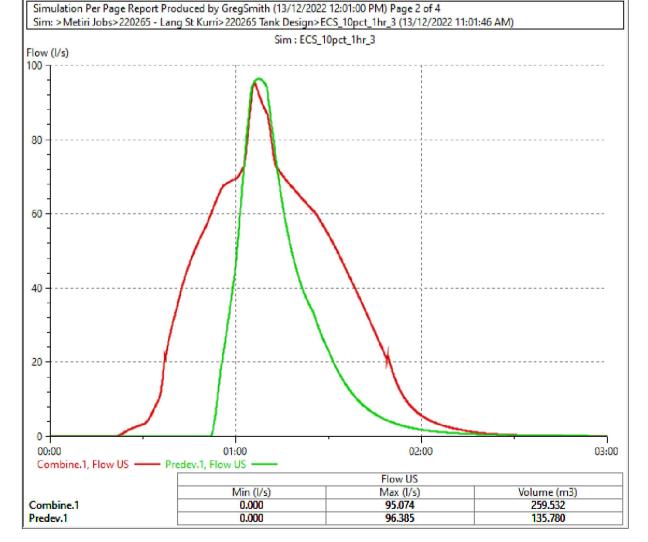
Max (I/s) 60.577

Simulation Per Page Report Produced by GregSmith (13/12/2022 11:59:38 AM) Page 1 of 4
Sim: > Metiri Jobs> 220265 - Lang St Kurri> 220265 Tank Design> ECS_20pct_1hr_8 (13/12/2022 11:01:46 AM)

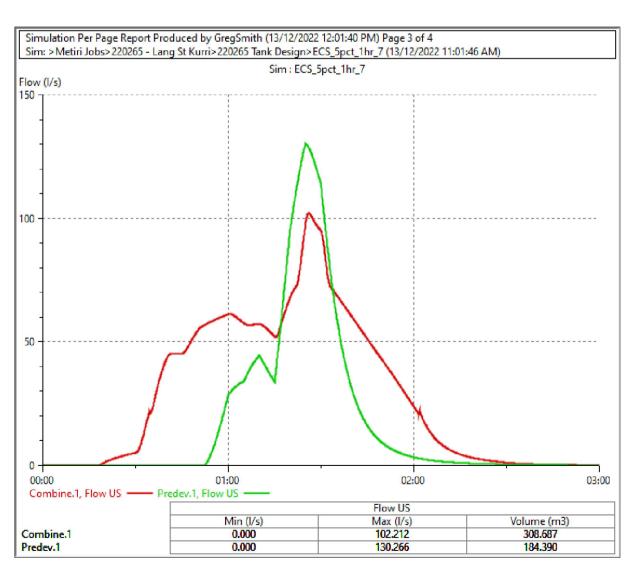
Combine.1, Flow US — Predev.1, Flow US —

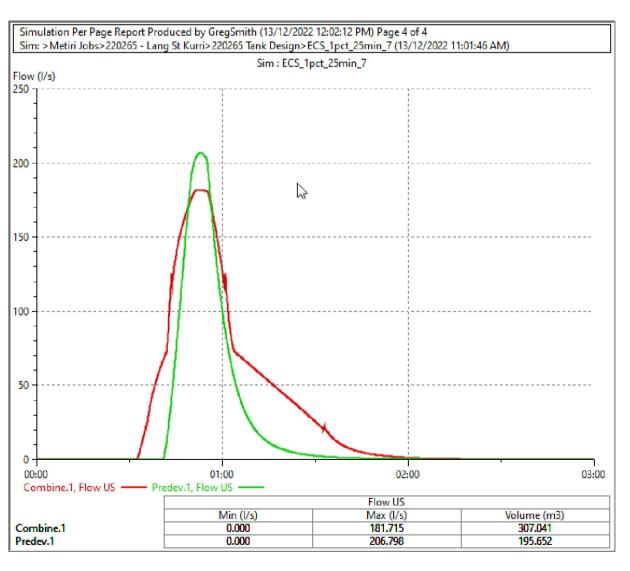
Combine.1

0.000



STORM ENSEMBLES





HYDROGRAPHS

CONCEPT ISSUE
NOT FOR CONSTRUCTION

REVISION

REV.	REVISION DETAILS	DES	DWN	СНК	DATE	N
0	DRAFT ISSUE FOR REVIEW	JRR	МНМ	JRR	08/12/22	
1	UPDATED FOR DA SUBMISSION	JRR	МНМ	JRR	13/12/22	
2	UPDATES TO PROPOSED LANE WORKS	JRR	МНМ	JRR	15/08/23	
						一

METIRI

DATUM SCALE

AHD

Volume (m3) 211.438



CIVIL ENGINEERING PLANS WOOLWORTHS KURRI KURRI LOT 136 DP 869710 174-178 LANG STREET, KURRI KURRI

ONSITE DETENTION CALCULATIONS

TOTAL SHEETS

09

VOTRAINT NO 124 PTY LTD

SHEET NUMBER

GUIDELINES.

INPUT PARAMETERS FOR THE MUSIC MODEL ARE PROVIDED IN THE TABLE

MUSIC MODEL INPUT PARAMETERS.

	AREA (ha)	% IMPERVIOUS
SITE	0.570	98
ROOF	0.439	100
CAR PARK	0.094	100
BALANCE	0.037	64

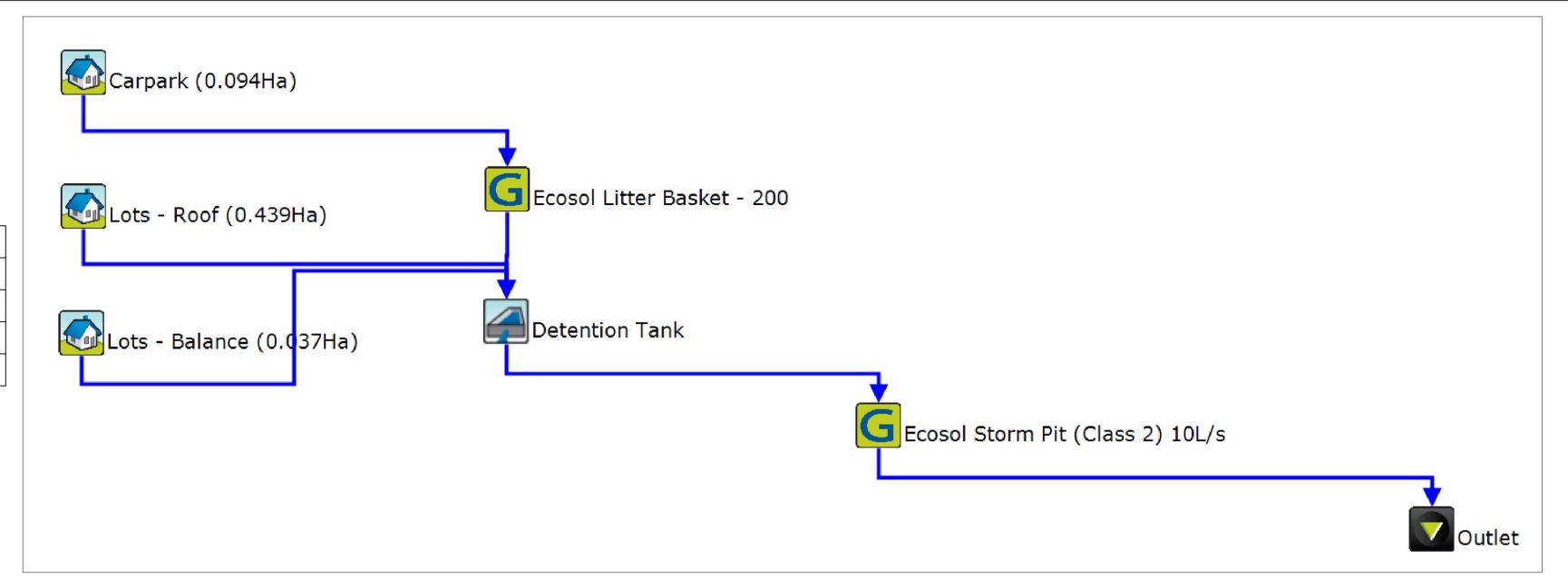
THE PROPOSED DEVELOPMENT IS REQUIRED TO TREAT STORMWATER PRIOR TO DISCHARGING FROM THE SITE. COUNCIL'S TREATMENT TARGETS, REPRESENTED BY REDUCTION %, ARE OUTLINED BELOW:

- GROSS POLLUTANTS (GP) 70%.
- TOTAL SUSPENDED SOLIDS (TSS) 80%
- TOTAL PHOSPHORUS (TP) 45%
- TOTAL NITROGEN (TN) 45%
- IT IS PROPOSED TO ACHIEVE THESE TREATMENT TARGETS BY ROUTING STORMWATER THROUGH A SERIES OF WATER QUALITY TREATMENT DEVICES CONSISTING OF:
- PROPRIETARY PIT INSERTS WITHIN SURFACE ENTRY STORMWATER PITS.
- UNDERGROUND DETENTION TANK.
- ECOSOL STORM PIT CLASS 2 (OR APPROVED EQUIVALENT) STORMWATER TREATMENT DEVICE.
- A MUSIC MODEL WAS CREATED TO DETERMINE THE SIZING AND CONFIGURATION OF THE WATER QUALITY TREATMENT DEVICES. A SCREENSHOT OF THE MODEL IS SHOWN TO THE RIGHT.
- SEVERAL CONFIGURATIONS OF THE TREATMENT DEVICES WERE TESTED WITH THE OPTIMAL CONFIGURATION TO MEET THE REQUIRED TREATMENT TARGETS DETAILED ON THE PREVIOUS SHEETS.
- RESULTS OF THE MUSIC MODEL DEMONSTRATES THE PROPOSED TREATMENT TRAIN EXCEEDS THE POLLUTANT REMOVAL TARGETS REQUIRED BY COUNCIL. A SUMMARY OF THE MUSIC MODEL RESULTS ARE PROVIDED IN THE TABLE BELOW.

MUSIC MODEL RESULTS

POLLUTANT	REDUCTION (%)			
POLLOTANT	MINIMUM	ACHIEVED		
GROSS POLLUTANTS (GP)	70.0	100		
TOTAL SUSPENDED SOLIDS (TSS)	80.0	93.46		
TOTAL PHOSPHORUS (TP)	45.0	69.83		
TOTAL NITROGEN (TN)	45.0	59.27		

A SCREENSHOT OF THE TREATMENT TRAIN EFFECTIVENESS REPORT FOR THE RECEIVING NODE IS SHOWN TO THE RIGHT.



MUSIC MODEL

		Sources	Residual Load	% Reduction
	Flow (ML/yr)	0.006739	0.003102	53.98
	Total Suspended Solids (kg/yr)	0.6218	0.04064	93.46
	Total Phosphorus (kg/yr)	0.00157	0.0004736	69.83
	Total Nitrogen (kg/yr)	0.01549	0.00631	59.27
	Gross Pollutants (kg/yr)	0.2596	0	100

TREATMENT TRAIN EFFECTIVENESS REPORT (RECEIVING NODE).

CONCEPT ISSUE
NOT FOR CONSTRUCTION

REVISION

REV. REVISION DETAILS

DES DWN CHK DATE

O DRAFT ISSUE FOR REVIEW

JRR MHM JRR 08/12/22

1 UPDATED FOR DA SUBMISSION

JRR MHM JRR 13/12/22

2 UPDATES TO PROPOSED LANE WORKS

JRR MHM JRR 15/08/23

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SCALE



CIVIL ENGINEERING PLANS WOOLWORTHS KURRI KURRI LOT 136 DP 869710 174-178 LANG STREET, KURRI KURRI

STORMWATER QUALITY TREATMENT CALCULATIONS

TOTAL SHEETS

09

CLIENT PROJECT NUMBER SHEET NUMBER

VOTRAINT NO 124 PTY LTD 220265 08

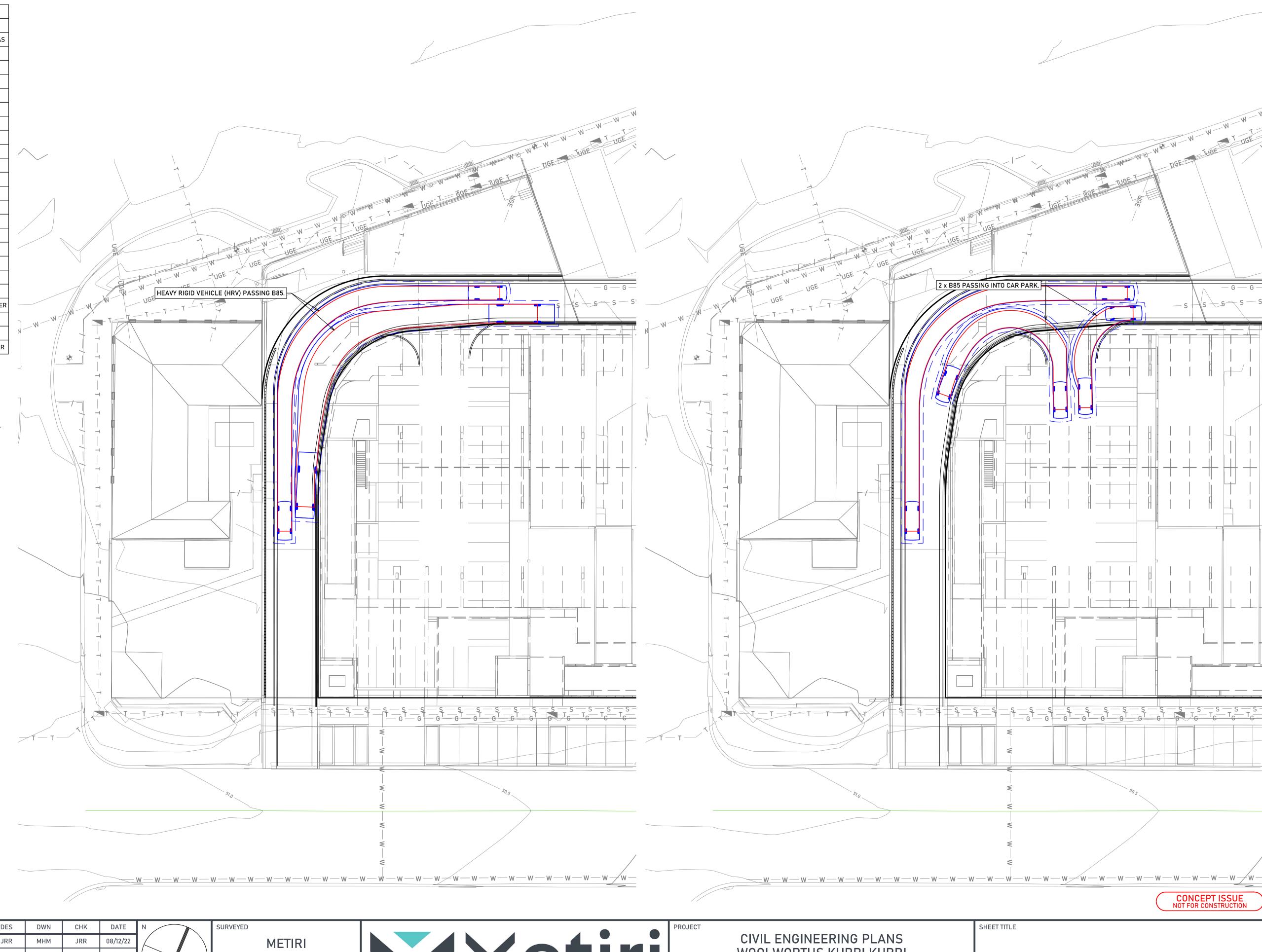
SHEET TITLE

	PROPOSED CONCRETE AREAS		
	PROPOSED AC AREAS		
	PROPOSED LANDSCAPED/UNDEVELOPED AREA		
	SURFACE CONTOURS (EXISTING/PROPOSED)		
+10.000E +10.000	SURFACE LEVEL (EXISTING/PROPOSED)		
5.00%E 5.00%	SURFACE SLOPE (EXISTING/PROPOSED)		
	RETAINING WALL (EXISTING/PROPOSED)		
- s - s - s - s -	EXISTING SEWER MAINS		
— W — W — W — W —	EXISTING WATER MAINS		
—— OHP —— OHP ———	EXISTING OVERHEAD ELECTRICITY		
—— UGE —— UGE ———	EXISTING UNDERGROUND ELECTRICITY		
-T - T - T - T - T	EXISTING TELECOMMUNICATIONS		
— G — G — G —	EXISTING GAS MAINS		
	STORMWATER PIPE (EXISTING/PROPOSED)		
	KERB INLET PIT (EXISTING/PROPOSED)		
	GRATED PIT (EXISTING/PROPOSED)		
	SEALED PIT (EXISTING/PROPOSED)		
	HEADWALL OUTLET (EXISTING/PROPOSED)		
-<<	TABLE DRAIN/SWALE (EXISTING/PROPOSED)		
	FORWARD MOVEMENT - WHEEL PATH		
	FORWARD MOVEMENT - BODY PATH		
	FORWARD MOVEMENT - BODY PATH 0.5m BUFFE		
	REVERSE MOVEMENT - WHEEL PATH		
	REVERSE MOVEMENT - BODY PATH		
	REVERSE MOVEMENT - BODY PATH 0.5m BUFFE		

NOTES

VEHICLE MOVEMENTS

- VEHICLE MOVEMENTS ILLUSTRATED ON PLANS HAVE BEEN MODELLED USING AUTODESK VEHICLE TRACKER AND REPRESENT THE:
- B85 TEMPLATE IN ACCORDANCE WITH AS2890.1.
- HEAVY RIGID VEHICLE (HRV) TEMPLATE IN ACCORDANCE WITH AS2890.2.





REV.	REVISION DETAILS	DES	DWN	СНК	DATE	N	SURVEYED	
0	DRAFT ISSUE FOR REVIEW	JRR	МНМ	JRR	08/12/22		METIRI	
1	UPDATED FOR DA SUBMISSION	JRR	МНМ	JRR	13/12/22			
2	UPDATES TO PROPOSED LANE WORKS	JRR	МНМ	JRR	15/08/23		DATUM	SCALE
							AHD	1:250
						12.50	0	6.25 12.50
						12.50		6.25
							SCALE @ A1	



WOOLWORTHS KURRI KURRI VEHICLE MOVEMENTS PLAN LOT 136 DP 869710 174-178 LANG STREET, KURRI KURRI PROJECT NUMBER SHEET NUMBER VOTRAINT NO 124 PTY LTD 220265

TOTAL SHEETS

09

REVISION